Well-balanced shock-capturing hybrid finite volume-finite difference schemes for Boussinesq-type models

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16 September 2010





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 - Limitation: Not applicable for wave propagation in intermediate/deeper waters (dispersion has an effect on free surface flow)



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- * Most popular (applied): Nonlinear Shallow Water Equations (SWE)
 - Limitation: Not applicable for wave propagation in intermediate/deeper waters (dispersion has an effect on free surface flow)
- * Use of popular Boussinesq-type models (but in conservation law from)
 - Nowgu's equations (Nowgu, 1993)
 - Madsen and Sörensen's (MS) equations (Madsen and Sörensen, 1992)

Both have good linear accuracy to $kd \approx 3$ (intermediate water).



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 η : free surface elevation b: topography d: steel water level $H = \eta + d$: total water depth

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 - Two-layer equation models (Lynnet et al., 2006-2010).

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Using $z_a = 0.53753d$ as optimal reference depth (Roeber et al., 2010).

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- 5. properly incorporate friction terms



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- Special treatment wet/dry fronts:
 - * Identify dry cells: through an adaptive (grid dependant) tolerance parameter
 - * Consistent depth reconstruction: satisfy $\frac{\partial h}{\partial x} = -\frac{\partial b}{\partial x}$ to high-order on wet/dry fronts
 - * Satisfy an extended *C*-property: Redefinition of the bed slope, numerical fluxes are computed assuming temporarily zero velocity at wet/dry faces (Brufau et al., 2004)

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- **Time Integration** (should at least match the order of truncation errors from dispersion terms): Third order Adams-Basforth predictor and fourth-order Adams-Moulton corrector stage (but also tested 3rd and 4th order Runge-Kutta methods).



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 $(Hu)_x$ used as indicator consistent with the conservative formulation, better detection of hydraulic jumps, U_1 and U_2 flow speeds used for breaking detection



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* Idea: Boussinesq degenerate into NSWE as dispersive terms become negligible (Tonelli and Petti, 2009)

If $\epsilon = \frac{A}{d} \leq 0.8$ Boussinesq are solved otherwise SWE are solved.

Some Numerical Tests and Results

Head on collision of two solitary waves



Surface profiles at times $t\sqrt{g/d}=0$, 56.63, 101.2 and 200.



Solitary wave run-up on a plane beach (Synolakis, 1987)

 $L = [-10, 100m], \quad \Delta x/d = 0.1, \quad CFL = 0.2, \quad n_m = 0.01, \quad \text{slope=1}: 19.85$ * First case: A/d = 0.04 (non-breaking)

* Second case: A/d = 0.28 (breaking)



Case A: Surface profiles at times $t\sqrt{g/d} = 20, 26, 32, 38$





Case A: Surface profiles at times $t\sqrt{g/d} = 44, 50, 56, 62$



Case B: Surface profiles at times $t\sqrt{g/d} = 10, 15, 20, 25$





Case B: Surface profiles at times $t\sqrt{g/d} = 30, 45, 55, 70$





Solitary wave propagation over reefs



Laboratory experiments at the O.H. Hinsdale Wave Research Laboratory of Oregon State University, 2007-2009 (Roeber et al., 2010).



 $L = [0, 48.8m], d = 1.0m, \Delta x/d = 0.1m, CFL = 0.4, n_m = 0.012, A/d = 0.5$





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- 2D extension of the present model
- Extend to other Boussinesq-type models



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THANK YOU FOR YOUR ATTENTION !

